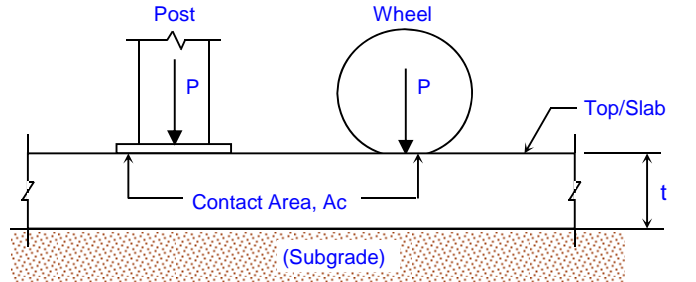


**CONCRETE SLAB ON GRADE ANALYSIS**  
**For Slab Subjected to Interior Concentrated Post or Wheel Loading**  
**Assuming ACI-360 "Type B" Design - Reinforced for Shrinkage and Temperature Only**

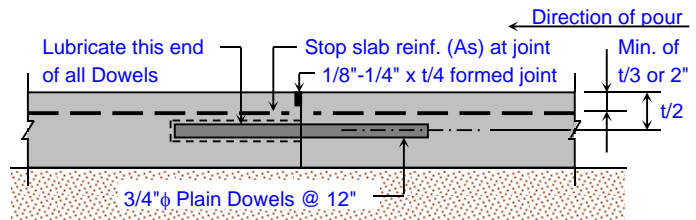
Job Name:	Sunset Farms, Inc. Feed Pad	Subject:	JLG G9-43A Telehandler
Job Number:	14774000	Originator:	JRH
		Checker:	

**Input Data:**

Slab Thickness, t =	8.000	in.
Concrete Strength, f'c =	4000	psi
Conc. Unit Weight, wc =	150	pcf
Reinforcing Yield, fy =	60000	psi
Subgrade Modulus, k =	200	pci
Concentrated Load, P =	8550.00	lbs.
Contact Area, Ac =	54.00	in.^2
Factor of Safety, FS =	2.00	
Dowel Bar Dia., db =	1.000	in.
Dowel Bar Spacing, s =	15.000	in.
Const. Joint Width, z =	0.1250	in.
Joint Spacing, L =	125.000	ft.
Temperature Range, ΔT =	80.00	deg.
Increase for 2nd Wheel, i =	0	%



**Concrete Slab on Grade**



**Typical Construction Joint for Load Transfer**

**Results:**

**Check Slab Flexural Stress:**

Effective Load Radius, a =	4.146	in.
Modulus of Elasticity, Ec =	3834254	psi
Modulus of Rupture, MR =	569.21	psi
Cracking Moment, Mr =	6.07	ft-k/ft.
Poisson's Ratio, μ =	0.15	
Radius of Stiffness, Lr =	30.245	in.
Equivalent Radius, b =	4.166	in.
1 Load: fb1(actual) =	190.60	psi
2 Loads: fb2(actual) =	N.A.	psi
Fb(allow) =	284.60	psi

(assuming unreinforced slab with interior load condition)

$a = \text{SQRT}(A_c/\pi)$   
 $E_c = 33 \cdot w_c^{1.5} \cdot \text{SQRT}(f'c)$   
 $MR = 9 \cdot \text{SQRT}(f'c)$   
 $Mr = MR \cdot (12 \cdot t^2/6) / 12000$  (per 1' = 12" width)  
 $\mu = 0.15$  (assumed for concrete)  
 $L_r = (E_c \cdot t^3 / (12 \cdot (1 - \mu^2) \cdot k))^{0.25}$   
 $b = \text{SQRT}(1.6 \cdot a^2 + t^2) - 0.675 \cdot t$ , for  $a < 1.724 \cdot t$   
 $fb1(\text{actual}) = 3 \cdot P \cdot (1 + \mu) / (2 \cdot \pi \cdot t^2) \cdot (\text{LN}(L_r/b) + 0.6159)$  (Ref. 1)  
 $fb2(\text{actual}) = \text{N.A.}$   
 $Fb(\text{allow}) = MR/FS$       **Fb(allow) >= fb(actual), O.K.**

**Note: Effect of a 2nd load was not considered.**

**Check Slab Bearing Stress:**

fp(actual) =	158.33	psi
Fp(allow) =	2390.68	psi

(assuming working stress) (Ref. 4)

$fp(\text{actual}) = P/A_c$   
 $Fp(\text{allow}) = 4.2 \cdot MR$       **Fp(allow) >= fp(actual), O.K.**

**Check Slab Punching Shear Stress:**

bo =	29.394	in.
fv(actual) =	17.41	psi
Fv(allow) =	153.69	psi

(assuming working stress) (Ref. 4)

$bo = 4 \cdot \text{SQRT}(A_c)$  (assumed shear perimeter)  
 $fv(\text{actual}) = P / (t \cdot (bo + 4 \cdot t))$   
 $Fv(\text{allow}) = 0.27 \cdot MR$       **Fv(allow) >= fv(actual), O.K.**

**Shrinkage and Temperature Reinf.:**

Friction Factor, F =	1.50	
Slab Weight, W =	100.00	psf
Reinf. Allow. Stress, fs =	45000	psi
As =	0.208	in.^2/ft.

(assuming subgrade drag method) (Ref. 3)

$F = 1.5$  (assumed friction factor between subgrade and slab)  
 $W = w_c \cdot (t/12)$   
 $fs = 0.75 \cdot fy$   
 $As = F \cdot L \cdot W / (2 \cdot fs)$

(continued)

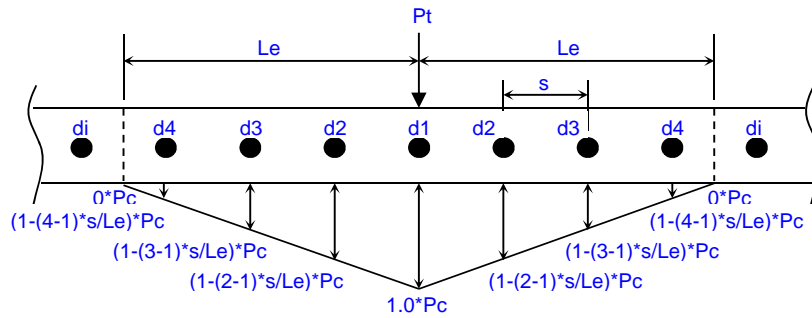
**Determine Estimated Crack Width:**

Slab-base Frict. Adjust., C =	1.00
Thermal Expansion, $\alpha$ =	0.0000055 in./in./deg
Shrinkage Coefficient, $\epsilon$ =	0.00035 in./in.
Est. Crack Width, $\Delta L$ =	1.1850 in.

(assuming no use of stabilized or granular subbase)  
 $C = 1.0$  (assumed value for no subbase)  
 $\alpha = 5.5 \times 10^{-6}$  (assumed thermal expansion coefficient)  
 $\epsilon = 3.5 \times 10^{-4}$  (assumed coefficient of shrinkage)  
 $\Delta L = C * L * 12 * (\alpha * \Delta T + \epsilon)$

**Check Bearing Stress on Dowels at Construction Joints with Load Transfer:**

(Ref. 2)



**Assumed Load Transfer Distribution for Dowels at Construction Joint**

Effective Length, $L_e$ =	30.245 in.
Effective Dowels, $N_e$ =	2.02 bars
Joint Load, $P_t$ =	4275.00 lbs.
Critical Dowel Load, $P_c$ =	2111.82 lbs.
Mod. of Dowel Suppt., $k_c$ =	1500000 psi
Mod. of Elasticity, $E_b$ =	29000000 psi
Inertia/Dowel Bar, $I_b$ =	0.0491 in. <sup>4</sup>
Relative Bar Stiffness, $\beta$ =	0.716
$f_d(\text{actual})$ =	3161.38 psi
$F_d(\text{allow})$ =	4000.00 psi

$L_e = 1.0 * L_r$  = applicable dist. each side of critical dowel  
 $N_e = 1.0 + 2 * \sum (1 - d(n-1) * s / L_e)$  (where: n = dowel #)  
 $P_t = 0.50 * P$  (assumed load transferred across joint)  
 $P_c = P_t / N_e$   
 $k_c = 1.5 \times 10^6$  (assumed for concrete)  
 $E_b = 29 \times 10^6$  (assumed for steel dowels)  
 $I_b = \pi * d_b^4 / 64$   
 $\beta = (k_c * d_b / (4 * E_b * I_b))^{1/4}$   
 $f_d(\text{actual}) = k_c * (P_c * (2 + \beta * z) / (4 * \beta^3 * E_b * I_b))$   
 $F_d(\text{allow}) = (4 - d_b) / 3 * f'_c$   **$F_d(\text{allow}) \geq f_d(\text{actual})$ , O.K.**

**References:**

- "Load Testing of Instrumented Pavement Sections - Improved Techniques for Applying the Finite Element Method to Strain Prediction in PCC Pavement Structures" - by University of Minnesota, Department of Civil Engineering (submitted to MN/DOT, March 24, 2002)
- "Dowel Bar Optimization: Phases I and II - Final Report" - by Max L. Porter (Iowa State University, 2001)
- "Design of Slabs on Grade" - ACI 360R-92 - by American Concrete Institute (from ACI Manual of Concrete Practice, 1999)
- "Slab Thickness Design for Industrial Concrete Floors on Grade" (IS195.01D) - by Robert G. Packard (Portland Cement Association, 1976)

**Comments:**