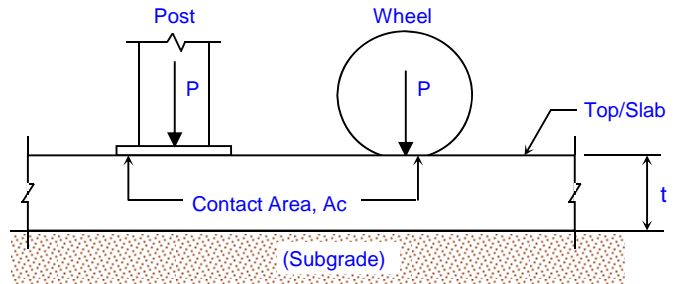


CONCRETE SLAB ON GRADE ANALYSIS
For Slab Subjected to Interior Concentrated Post or Wheel Loading
Assuming ACI-360 "Type B" Design - Reinforced for Shrinkage and Temperature Only

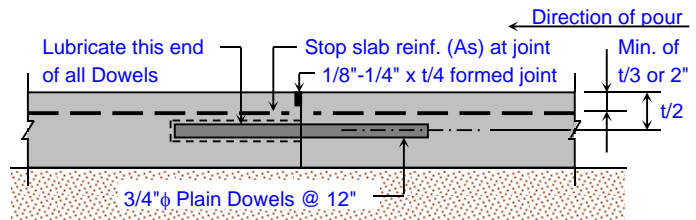
Job Name:	Sunset Farms, Inc. Feed Pad	Subject:	Pay Loader - Deere 544K High Lift
Job Number:	14774000	Originator:	JRH
		Checker:	

Input Data:

Slab Thickness, t =	8.000	in.
Concrete Strength, f'c =	4000	psi
Conc. Unit Weight, wc =	150	pcf
Reinforcing Yield, fy =	60000	psi
Subgrade Modulus, k =	200	pci
Concentrated Load, P =	10115.00	lbs.
Contact Area, Ac =	86.00	in.^2
Factor of Safety, FS =	2.00	
Dowel Bar Dia., db =	1.000	in.
Dowel Bar Spacing, s =	15.000	in.
Const. Joint Width, z =	0.1250	in.
Joint Spacing, L =	125.000	ft.
Temperature Range, ΔT =	80.00	deg.
Increase for 2nd Wheel, i =	0	%



Concrete Slab on Grade



Typical Construction Joint for Load Transfer

Results:

Check Slab Flexural Stress:

Effective Load Radius, a =	5.232	in.
Modulus of Elasticity, Ec =	3834254	psi
Modulus of Rupture, MR =	569.21	psi
Cracking Moment, Mr =	6.07	ft-k/ft.
Poisson's Ratio, μ =	0.15	
Radius of Stiffness, Lr =	30.245	in.
Equivalent Radius, b =	4.983	in.
1 Load: fb1(actual) =	209.95	psi
2 Loads: fb2(actual) =	N.A.	psi
Fb(allow) =	284.60	psi

(assuming unreinforced slab with interior load condition)
 $a = \text{SQRT}(Ac/\pi)$
 $E_c = 33 \cdot wc^{1.5} \cdot \text{SQRT}(f'c)$
 $MR = 9 \cdot \text{SQRT}(f'c)$
 $Mr = MR \cdot (12 \cdot t^2 / 6) / 12000$ (per 1' = 12" width)
 $\mu = 0.15$ (assumed for concrete)
 $L_r = (E_c \cdot t^3 / (12 \cdot (1 - \mu^2) \cdot k))^{0.25}$
 $b = \text{SQRT}(1.6 \cdot a^2 + t^2) - 0.675 \cdot t$, for $a < 1.724 \cdot t$
 $fb1(\text{actual}) = 3 \cdot P \cdot (1 + \mu) / (2 \cdot \pi \cdot t^2) \cdot (\text{LN}(L_r/b) + 0.6159)$ (Ref. 1)
 $fb2(\text{actual}) = \text{N.A.}$
 $Fb(\text{allow}) = MR/FS$ **Fb(allow) >= fb(actual), O.K.**
Note: Effect of a 2nd load was not considered.

Check Slab Bearing Stress:

fp(actual) =	117.62	psi
Fp(allow) =	2390.68	psi

(assuming working stress) (Ref. 4)
 $fp(\text{actual}) = P/Ac$
 $Fp(\text{allow}) = 4.2 \cdot MR$ **Fp(allow) >= fp(actual), O.K.**

Check Slab Punching Shear Stress:

bo =	37.094	in.
fv(actual) =	18.30	psi
Fv(allow) =	153.69	psi

(assuming working stress) (Ref. 4)
 $bo = 4 \cdot \text{SQRT}(Ac)$ (assumed shear perimeter)
 $fv(\text{actual}) = P / (t \cdot (bo + 4 \cdot t))$
 $Fv(\text{allow}) = 0.27 \cdot MR$ **Fv(allow) >= fv(actual), O.K.**

Shrinkage and Temperature Reinf.:

Friction Factor, F =	1.50	
Slab Weight, W =	100.00	psf
Reinf. Allow. Stress, fs =	45000	psi
As =	0.208	in.^2/ft.

(assuming subgrade drag method) (Ref. 3)
 $F = 1.5$ (assumed friction factor between subgrade and slab)
 $W = wc \cdot (t/12)$
 $fs = 0.75 \cdot fy$
 $As = F \cdot L \cdot W / (2 \cdot fs)$

(continued)

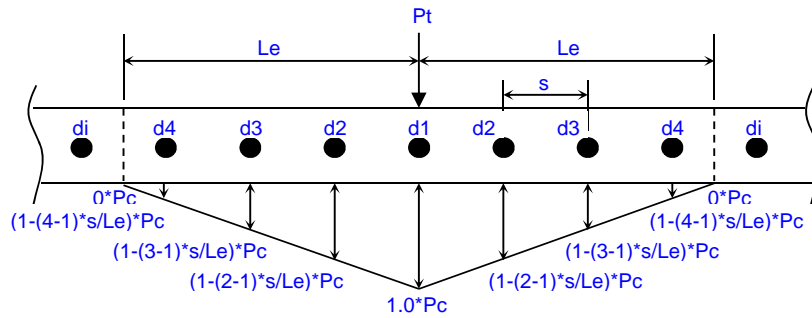
Determine Estimated Crack Width:

Slab-base Frict. Adjust., C =	1.00
Thermal Expansion, α =	0.0000055 in./in./deg
Shrinkage Coefficient, ϵ =	0.00035 in./in.
Est. Crack Width, ΔL =	1.1850 in.

(assuming no use of stabilized or granular subbase)
 $C = 1.0$ (assumed value for no subbase)
 $\alpha = 5.5 \times 10^{-6}$ (assumed thermal expansion coefficient)
 $\epsilon = 3.5 \times 10^{-4}$ (assumed coefficient of shrinkage)
 $\Delta L = C * L * 12 * (\alpha * \Delta T + \epsilon)$

Check Bearing Stress on Dowels at Construction Joints with Load Transfer:

(Ref. 2)



Assumed Load Transfer Distribution for Dowels at Construction Joint

Effective Length, L_e =	30.245	in.
Effective Dowels, N_e =	2.02	bars
Joint Load, P_t =	5057.50	lbs.
Critical Dowel Load, P_c =	2498.37	lbs.
Mod. of Dowel Suppt., k_c =	1500000	psi
Mod. of Elasticity, E_b =	29000000	psi
Inertia/Dowel Bar, I_b =	0.0491	in. ⁴
Relative Bar Stiffness, β =	0.716	
$f_d(\text{actual})$ =	3740.04	psi
$F_d(\text{allow})$ =	4000.00	psi

$L_e = 1.0 * L_r$ = applicable dist. each side of critical dowel
 $N_e = 1.0 + 2 * \sum (1 - d(n-1) * s / L_e)$ (where: n = dowel #)
 $P_t = 0.50 * P$ (assumed load transferred across joint)
 $P_c = P_t / N_e$
 $k_c = 1.5 \times 10^6$ (assumed for concrete)
 $E_b = 29 \times 10^6$ (assumed for steel dowels)
 $I_b = \pi * d_b^4 / 64$
 $\beta = (k_c * d_b / (4 * E_b * I_b))^{1/4}$
 $f_d(\text{actual}) = k_c * (P_c * (2 + \beta * z) / (4 * \beta^3 * E_b * I_b))$
 $F_d(\text{allow}) = (4 - d_b) / 3 * f'_c$ **$F_d(\text{allow}) \geq f_d(\text{actual})$, O.K.**

References:

- "Load Testing of Instrumented Pavement Sections - Improved Techniques for Applying the Finite Element Method to Strain Prediction in PCC Pavement Structures" - by University of Minnesota, Department of Civil Engineering (submitted to MN/DOT, March 24, 2002)
- "Dowel Bar Optimization: Phases I and II - Final Report" - by Max L. Porter (Iowa State University, 2001)
- "Design of Slabs on Grade" - ACI 360R-92 - by American Concrete Institute (from ACI Manual of Concrete Practice, 1999)
- "Slab Thickness Design for Industrial Concrete Floors on Grade" (IS195.01D) - by Robert G. Packard (Portland Cement Association, 1976)

Comments: